

AS3020: Aerospace Structures Module 7: Elastic Stability

Instructor: Nidish Narayanaa Balaji

Dept. of Aerospace Engg., IIT-Madras, Chennai

October 22, 2024

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 \leftarrow \Box

1. [Introduction](#page-2-0)

- The key intuition for elastic stability comes from analyzing the quantity $U - \Pi$ around its extrema.
	- Maxima in $U \Pi$ correspond to **unstable solutions**;
	- Minima in $U \Pi$ correspond to stable solutions.
- Investigating the second derivative $(U \Pi)$ ("Hessian") of the quantity allows for efficient classification;
- In 1D $(u \in \mathbb{R})$, the sign of $\frac{\partial (U \Pi)}{\partial u^2}$ is sufficient for this;
- In higher dimensions, we obtain an eigenvalue problem.

 \leftarrow \Box \rightarrow

1.1. [Column Buckling](#page-3-0)

[Introduction](#page-2-0)

We already derived the governing equations for a beam under uniform axial stress $\frac{P}{A}$. When this is compressive, the governing equation can be written as

$$
E I v^{\prime\prime\prime\prime} + P v^{\prime\prime} = 0.
$$

We showed in class that this can be used to recover Euler's Critical Loads,

$$
P_n = n^2 \frac{\pi^2 EI}{\ell^2}, \quad v(X_1) = V \sin\left(n \frac{\pi X_1}{\ell}\right).
$$

We solved a Sturm-Liouville Problem to obtain these.

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We will now derive the governing equations of thin plates with the Kirchhoff-Love Plate Theory, which is the simplest generalization of Euler-Bernoulli Beam Theory.

Euler-Bernoulli Beams

- Sections *move* rigidly;
- **Plane** sections remain perpendicular to the centroidal axis.

KL Plates

- Line elements along thickness move rigidly;
- Line elements remain perpendicular to the mid-plane.
- The above assumptions lead to the zeroing out of certain strains in the formulation that leads to a simplified kinematic description. For plates this is,

$$
u_1 = -X_3 w_{,1}
$$

\n
$$
u_2 = -X_3 w_{,2}
$$

\n
$$
u_3 = w,
$$

where w is a function of X_1, X_2 .

 $e₂$

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Variational Approach for Derivation

Using the kinematic description we write out the strains (linear and nonlinear) as

$$
E_{11} = u_{1,1} + \frac{1}{2} (u_{1,1}^2 + u_{2,1}^2 + u_{3,1}^2)
$$

= $-X_3 w_{,11} + \frac{1}{2} (X_3^2 w_{,11}^2 + X_3^2 w_{,12}^2 + w_{,1}^2)$

$$
E_{22} = u_{2,2} + \frac{1}{2} (u_{1,2}^2 + u_{2,2}^2 + u_{3,2}^2)
$$

= $-X_3 w_{,22} + \frac{1}{2} (X_3^2 w_{,12}^2 + X_3^2 w_{,22}^2 + w_{,2}^2)$

$$
\gamma_{12} = u_{1,2} + u_{2,1} + (u_{1,1}u_{1,2} + u_{2,1}u_{2,2} + u_{3,1}u_{3,2})
$$

= $-2X_3 w_{,12} + (X_3^2 w_{,11} w_{,12} + X_3^2 w_{,12} w_{,22} + w_{,1}w_{,2}),$

where the nonlinear (quadratic) terms are highlighted in blue.

• Just like in the case of the beam, we retain only the quadratic terms for the internal energy. \leftarrow \Box

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Bending Strain Energy under Plane Stress

We have to first write down the stresses before the energy can be expressed. Under plane stress assumptions we get,

$$
\begin{bmatrix} E_{11} \\ E_{12} \\ \gamma_{12} \end{bmatrix} = \frac{1}{E} \begin{bmatrix} 1 & -\nu & 0 \\ -\nu & 1 & 0 \\ 0 & 0 & 2(1+\nu) \end{bmatrix} \begin{bmatrix} \sigma_{11} \\ \sigma_{22} \\ \sigma_{12} \end{bmatrix}
$$

$$
\implies \begin{bmatrix} \sigma_{11} \\ \sigma_{22} \\ \sigma_{12} \end{bmatrix} = \frac{E}{1-\nu^2} \begin{bmatrix} 1 & \nu & 0 \\ \nu & 1 & 0 \\ 0 & 0 & \frac{1-\nu}{2} \end{bmatrix} \begin{bmatrix} E_{11} \\ E_{12} \\ \gamma_{12} \end{bmatrix}
$$

The bending energy (up to $\mathcal{O}(v^2)$) is

$$
U_b = \int_{-\frac{t}{2}}^{\frac{t}{2}} \frac{1}{2} (\sigma_{11} E_{11} + \sigma_{22} E_{22} + \sigma_{12} \gamma_{12}) dX_3
$$

=
$$
\frac{1}{2} \underbrace{\frac{Et^3}{12(1-\nu^2)}}_{D} (w_{,11}^2 + w_{,22}^2 + 2(1-\nu)w_{,12}^2 + 2w_{,11}w_{,22})
$$

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Work Done by Axial Stresses

 \bullet We consider axial loads P_1, P_2, P_{12} as shown. The work done by these is contributed by the quadratic strains

$$
U_c = \frac{P_1}{24} \left(t^2 (w_{,11}^2 + w_{,12}^2) + 12w_{,1}^2 \right) + \frac{P_2}{24} \left(t^2 (w_{,12}^2 + w_{,22}^2) + 12w_{,2}^2 \right) + \frac{P_{12}}{12} \left(t^2 w_{,12} (w_{,11} + w_{,22}) + 12w_{,1}w_{,2} \right).
$$

We will ignore the t^2 terms in the above to give,

$$
U_c = \frac{1}{2} \left(P_1 w_{,1}^2 + P_2 w_{,2}^2 + 2 P_{12} w_{,1} w_{,2} \right).
$$

Other Loads

When there is also a distributed transverse load f acting, the load work done is given by

$$
\Pi=\int_{\mathcal{D}}fw dX_1 dX_2
$$

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2.1. [Principle of Virtual Work](#page-8-0)

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The total work done by the system is written as,

$$
\mathcal{L} = U_b + U_c - \Pi = \frac{D}{2} \left(w_{,11}^2 + w_{,22}^2 + 2(1 - \nu)w_{,12}^2 + 2w_{,11}w_{,22} \right) \n+ \frac{1}{2} \left(P_1 w_{,11}^2 + P_2 w_{,2}^2 + 2P_{12}w_{,1}w_{,2} \right) - fw
$$

The Euler-Lagrange Equations are written as:

$$
\frac{d^2}{dX_1^2}\frac{\partial \mathcal{L}}{\partial w_{,11}}+\frac{d^2}{dX_2^2}\frac{\partial \mathcal{L}}{\partial w_{,22}}+\frac{d^2}{dX_1dX_2}\frac{\partial \mathcal{L}}{\partial w_{,12}}-\frac{d}{dX_1}\frac{\partial \mathcal{L}}{\partial w_{,1}}-\frac{d}{dX_2}\frac{\partial \mathcal{L}}{\partial w_{,2}}+\frac{\partial \mathcal{L}}{\partial w}=0.
$$

This leads to,

 Et^3 $12(1-\nu^2)$ \sum_{D} D $(w_{,1111} + w_{,2222} + 2w_{,1122}) - (P_1w_{,11} + P_2w_{,22} + 2P_{12}w_{,12}) - f = 0$

 \leftarrow \Box \rightarrow

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The general plate equation can be interpreted in two ways just as before.

 $D(w_{,1111} + w_{,2222} + 2w_{,1122}) - (P_1w_{,11} + P_2w_{,22} + 2P_{12}w_{,12}) - f = 0$

Membranes

 \bullet When the quantity D is very small, the system is approximated well as

 $(P_1w_{.11} + P_2w_{.22} + 2P_{12}w_{.12}) + f = 0$

For the isotropic case shear-free case $(P_1 = P_2 = P, P_{12} = 0)$ we have,

 $P\nabla^2 w + f$

Plate Buckling

• For the $f = 0$ case undergoing compressive loading $(P_1 \rightarrow -P_1,$ $P_2 \rightarrow -P_2$ $P_{12} \rightarrow -P_{12}$, the governing equation is

 $D\nabla^4 w + (P_1 w_{,11} + P_2 w_{,22} + 2P_{12} w_{,12}) = 0.$

• This is a slightly more complicated Sturm-Liouville type problem than the one encountered with column buckling.

References I

[1] T. H. G. Megson. Aircraft Structures for Engineering Students, Elsevier, 2013. isbn: 978-0-08-096905-3 (cit. on p. [2\)](#page-1-0).