

AS3020: Aerospace Structures Module 7: Elastic Stability

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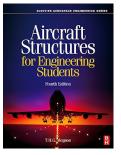
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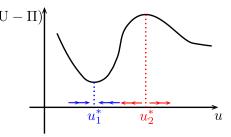


Chapters 7-9 in Megson [1]

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1. Introduction

- The key intuition for elastic stability comes from analyzing the quantity $U \Pi$ around its extrema.
 - Maxima in U Π correspond to **unstable solutions**;
 - Minima in $U \Pi$ correspond to stable solutions.
- Investigating the second derivative $(U \Pi)'$ ("Hessian") of the quantity allows for efficient classification;
- In 1D $(u \in \mathbb{R})$, the sign of $\frac{\partial (U-\Pi)}{\partial u^2}$ is sufficient for this;
- In higher dimensions, we obtain an eigenvalue problem.



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1.1. Column Buckling

Introduction

• We already derived the governing equations for a beam under uniform axial stress $\frac{P}{A}$. When this is compressive, the governing equation can be written as

$$EIv'''' + Pv'' = 0$$

• We showed in class that this can be used to recover Euler's Critical Loads,

$$P_n = n^2 \frac{\pi^2 EI}{\ell^2}, \quad v(X_1) = V \sin\left(n\frac{\pi X_1}{\ell}\right).$$

• We solved a **Sturm-Liouville Problem** to obtain these.

Plates

2. Plates

• We will now derive the governing equations of thin plates with the **Kirchhoff-Love Plate Theory**, which is the simplest generalization of **Euler-Bernoulli Beam Theory**.

Euler-Bernoulli Beams

- Sections *move* rigidly;
- Plane sections remain perpendicular to the centroidal axis.

KL Plates

- Line elements along thickness *move* rigidly;
- Line elements remain perpendicular to the mid-plane.
- The above assumptions lead to the zeroing out of certain strains in the formulation that leads to a simplified kinematic description. For plates this is, A^{ℓ_3}

$$u_1 = -X_3 w_{,1}$$

 $u_2 = -X_3 w_{,2}$
 $u_3 = w,$

where w is a function of X_1, X_2 .

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 e_2

2. Plates

Variational Approach for Derivation

• Using the kinematic description we write out the strains (linear and nonlinear) as

$$\begin{split} E_{11} &= u_{1,1} + \frac{1}{2} (u_{1,1}^2 + u_{2,1}^2 + u_{3,1}^2) \\ &= -X_3 w_{,11} + \frac{1}{2} \left(X_3^2 w_{,11}^2 + X_3^2 w_{,12}^2 + w_{,1}^2 \right) \\ E_{22} &= u_{2,2} + \frac{1}{2} (u_{1,2}^2 + u_{2,2}^2 + u_{3,2}^2) \\ &= -X_3 w_{,22} + \frac{1}{2} \left(X_3^2 w_{,12}^2 + X_3^2 w_{,22}^2 + w_{,2}^2 \right) \\ \gamma_{12} &= u_{1,2} + u_{2,1} + (u_{1,1} u_{1,2} + u_{2,1} u_{2,2} + u_{3,1} u_{3,2}) \\ &= -2X_3 w_{,12} + \left(X_3^2 w_{,11} w_{,12} + X_3^2 w_{,12} w_{,22} + w_{,1} w_{,2} \right), \end{split}$$

where the nonlinear (quadratic) terms are highlighted in blue.

• Just like in the case of the beam, we **retain only the quadratic terms** for the internal energy.

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2. Plates

Bending Strain Energy under Plane Stress

• We have to first write down the stresses before the energy can be expressed. Under **plane stress** assumptions we get,

$$\begin{bmatrix} E_{11} \\ E_{12} \\ \gamma_{12} \end{bmatrix} = \frac{1}{E} \begin{bmatrix} 1 & -\nu & 0 \\ -\nu & 1 & 0 \\ 0 & 0 & 2(1+\nu) \end{bmatrix} \begin{bmatrix} \sigma_{11} \\ \sigma_{22} \\ \sigma_{12} \end{bmatrix}$$
$$\Longrightarrow \begin{bmatrix} \sigma_{11} \\ \sigma_{22} \\ \sigma_{12} \end{bmatrix} = \frac{E}{1-\nu^2} \begin{bmatrix} 1 & \nu & 0 \\ \nu & 1 & 0 \\ 0 & 0 & \frac{1-\nu}{2} \end{bmatrix} \begin{bmatrix} E_{11} \\ E_{12} \\ \gamma_{12} \end{bmatrix}$$

• The bending energy (up to $\mathcal{O}(v^2)$) is

$$U_{b} = \int_{-\frac{t}{2}}^{\frac{t}{2}} \frac{1}{2} \left(\sigma_{11} E_{11} + \sigma_{22} E_{22} + \sigma_{12} \gamma_{12} \right) dX_{3}$$

= $\frac{1}{2} \underbrace{\frac{Et^{3}}{12(1-\nu^{2})}}_{D} \left(w_{,11}^{2} + w_{,22}^{2} + 2(1-\nu)w_{,12}^{2} + 2w_{,11}w_{,22} \right)$

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Plates

2. Plates

Work Done by Axial Stresses

• We consider axial loads P_1, P_2, P_{12} as shown. The work done by these is contributed by the quadratic strains

$$\begin{aligned} \mathbf{U}_{c} = & \frac{P_{1}}{24} \left(t^{2} (w_{,11}^{2} + w_{,12}^{2}) + 12w_{,1}^{2} \right) + \frac{P_{2}}{24} \left(t^{2} (w_{,12}^{2} + w_{,22}^{2}) + 12w_{,2}^{2} \right) \\ & + \frac{P_{12}}{12} \left(t^{2} w_{,12} (w_{,11} + w_{,22}) + 12w_{,1} w_{,2} \right). \end{aligned}$$

• We will ignore the t^2 terms in the above to give,

$$U_c = \frac{1}{2} \left(P_1 w_{,1}^2 + P_2 w_{,2}^2 + 2P_{12} w_{,1} w_{,2} \right).$$

Other Loads

When there is also a distributed transverse load f acting, the load work done is given by

$$\Pi = \int_{\mathcal{D}} fw dX_1 dX_2$$

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2.1. Principle of Virtual Work

Plates

• The total work done by the system is written as,

$$\mathcal{L} = U_b + U_c - \Pi = \frac{D}{2} \left(w_{,11}^2 + w_{,22}^2 + 2(1-\nu)w_{,12}^2 + 2w_{,11}w_{,22} \right) \\ + \frac{1}{2} \left(P_1 w_{,1}^2 + P_2 w_{,2}^2 + 2P_{12}w_{,1}w_{,2} \right) - fw$$

• The Euler-Lagrange Equations are written as:

$$\frac{d^2}{dX_1^2}\frac{\partial\mathcal{L}}{\partial w_{,11}} + \frac{d^2}{dX_2^2}\frac{\partial\mathcal{L}}{\partial w_{,22}} + \frac{d^2}{dX_1dX_2}\frac{\partial\mathcal{L}}{\partial w_{,12}} - \frac{d}{dX_1}\frac{\partial\mathcal{L}}{\partial w_{,1}} - \frac{d}{dX_2}\frac{\partial\mathcal{L}}{\partial w_{,2}} + \frac{\partial\mathcal{L}}{\partial w} = 0.$$

• This leads to,

 $\underbrace{\frac{Et^3}{12(1-\nu^2)}}_{D}(w_{,1111}+w_{,2222}+2w_{,1122}) - (P_1w_{,11}+P_2w_{,22}+2P_{12}w_{,12}) - f = 0$

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2.1. Principle of Virtual Work

Plates

• The general plate equation can be interpreted in two ways just as before.

 $D(w_{,1111} + w_{,2222} + 2w_{,1122}) - (P_1w_{,11} + P_2w_{,22} + 2P_{12}w_{,12}) - f = 0$

Membranes

• When the quantity *D* is very small, the system is approximated well as

 $(P_1w_{,11} + P_2w_{,22} + 2P_{12}w_{,12}) + f = 0$

• For the isotropic case shear-free case $(P_1 = P_2 = P, P_{12} = 0)$ we have,

 $P\nabla^2 w + f$

Plate Buckling

• For the f = 0 case undergoing compressive loading $(P_1 \rightarrow -P_1, P_2 \rightarrow -P_2 P_{12} \rightarrow -P_{12})$, the governing equation is

 $D\nabla^4 w + (P_1 w_{,11} + P_2 w_{,22} + 2P_{12} w_{,12}) = 0.$

• This is a slightly more complicated Sturm-Liouville type problem than the one encountered with column buckling.

References I

 T. H. G. Megson. Aircraft Structures for Engineering Students, Elsevier, 2013. ISBN: 978-0-08-096905-3 (cit. on p. 2).