

## AS3020: Aerospace Structures Module 5: Torsion of Beams

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Chapter 7 in Sun [1]



Chapters 3, 18, 19 in Megson [2]

# 1. Solid Section Torsion

Basic Setup



- We assume:
  - In the stresses of the stre
    - $\sigma_{11} = \sigma_{22} = \sigma_{33} = 0$
  - **2** Sections "rotate rigidly":
    - $\gamma_{23} = 0 \implies \sigma_{23} = 0.$
  - Body is at equilibrium under constant torque applied at right end.
- We will denote the section by S and the section-boundary by Γ.

Solid Section Torsion

• Since we assume  $\sigma_{11} = \sigma_{22} = \sigma_{33} = \sigma_{23} = 0$ , the equilibrium equations read,

$$\sigma_{12,2} + \sigma_{13,3} = 0, \quad \sigma_{12,1} = 0, \quad \sigma_{13,1} = 0.$$

• We introduce the **Prandtl Stress Function**  $\phi(X_2, X_3)$  (no dependence on  $X_1$ ) such that

$$\sigma_{12} = \phi_{,3}, \quad \sigma_{13} = -\phi_{,2}.$$

This satisfies equilibrium by definition.

- In terms of strains the above assumptions imply that we only have  $E_{12}$  and  $E_{13}$  active. **Recall** that Strain compatibility is  $\epsilon_{mjk}\epsilon_{nil}E_{ij,mn} = 0$  (see Module 3).
- The non-trivial compatibility equations read,

$$\begin{bmatrix} E_{12,23} - E_{13,22} &= 0\\ E_{12,33} - E_{13,23} &= 0 \end{bmatrix} \implies \begin{array}{c} \phi_{,332} + \phi_{,222} &= 0\\ \phi_{,333} + \phi_{,322} &= 0 \end{bmatrix} \implies \boxed{\nabla^2 \phi = \text{constant}}.$$

• This is known as the **Poisson's problem**. What about <u>Boundary</u> <u>Conditions</u>?

Solid Section Torsion

• Since we assume  $\sigma_{11} = \sigma_{22} = \sigma_{33} = \sigma_{23} = 0$ , the equilibrium equations read,

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• This is known as the **Poisson's problem**. What about <u>Boundary</u> Conditions?

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Solid Section Torsion



• We derive the coordinate transformation on the boundary as follows:

$$dX_{2}\underline{e}_{2} + dX_{3}\underline{e}_{3} = ds\underline{e}_{s} + dn\underline{e}_{n}$$

$$\implies \begin{bmatrix} dX_{2} \\ dX_{3} \end{bmatrix} = \begin{bmatrix} \langle \underline{e}_{2}, \underline{e}_{s} \rangle & \langle \underline{e}_{2}, \underline{e}_{n} \rangle \\ \langle \underline{e}_{3}, \underline{e}_{s} \rangle & \langle \underline{e}_{3}, \underline{e}_{n} \rangle \end{bmatrix} \begin{bmatrix} ds \\ dn \end{bmatrix}$$
and,
$$\begin{bmatrix} \underline{e}_{s} \\ \underline{e}_{n} \end{bmatrix} = \begin{bmatrix} \langle \underline{e}_{2}, \underline{e}_{s} \rangle & \langle \underline{e}_{3}, \underline{e}_{n} \rangle \\ \langle \underline{e}_{2}, \underline{e}_{n} \rangle & \langle \underline{e}_{3}, \underline{e}_{n} \rangle \end{bmatrix} \begin{bmatrix} \underline{e}_{2} \\ \underline{e}_{3} \end{bmatrix}$$

$$= \begin{bmatrix} X_{2,s} & X_{3,s} \\ X_{2,n} & X_{3,n} \end{bmatrix} \begin{bmatrix} \underline{e}_{2} \\ \underline{e}_{3} \end{bmatrix}$$

• Considering only Cartesian transformations (inverse has to be transpose), we will also have

$$\begin{bmatrix} \underline{e}_s \\ \underline{e}_n \end{bmatrix} = \begin{bmatrix} X_{3,n} & -X_{2,n} \\ -X_{3,s} & X_{2,s} \end{bmatrix} \begin{bmatrix} \underline{e}_2 \\ \underline{e}_3 \end{bmatrix}$$

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Solid Section Torsion



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and, 
$$\begin{bmatrix} \underline{e}_{s} \\ \underline{e}_{n} \end{bmatrix} = \begin{bmatrix} \langle \underline{e}_{2}, \underline{e}_{s} \rangle & \langle \underline{e}_{3}, \underline{e}_{n} \rangle \\ \langle \underline{e}_{2}, \underline{e}_{n} \rangle & \langle \underline{e}_{3}, \underline{e}_{n} \rangle \end{bmatrix} \begin{bmatrix} \underline{e}_{2} \\ \underline{e}_{3} \end{bmatrix}$$

$$\implies = \begin{bmatrix} X_{2,s} & X_{3,s} \\ X_{2,n} & X_{3,n} \end{bmatrix} \begin{bmatrix} \underline{e}_{2} \\ \underline{e}_{3} \end{bmatrix}$$

Considering only Cartesian transformations (inverse has to be transpose), we will also have

$$\begin{bmatrix} \underline{e}_s \\ \underline{e}_n \end{bmatrix} = \begin{bmatrix} X_{3,n} & -X_{2,n} \\ -X_{3,s} & X_{2,s} \end{bmatrix} \begin{bmatrix} \underline{e}_2 \\ \underline{e}_3 \end{bmatrix}$$

# 1.1. Derivation of Coordinate Transformation Relationships

Stress Formulation

• For cartesian transformations, the determinant has to be unity. So the inverse can be written as

$$\underbrace{\begin{bmatrix} X_{2,s} & X_{3,s} \\ X_{2,n} & X_{3,n} \end{bmatrix}^{-1}}_{\mathbb{T}^{-1}} = \underbrace{\begin{bmatrix} X_{3,n} & -X_{3,s} \\ -X_{2,n} & X_{2,s} \end{bmatrix}}_{Adj(\mathbb{T})}.$$

• Also, for cartesian transformations, the inverse has to be the transpose of the matrix. So we have

• So the following equalities make sense:  

$$\begin{bmatrix}
 \begin{bmatrix}
 X_{2,s} & X_{2,n} \\
 X_{3,s} & X_{3,n}
\end{bmatrix} = \begin{bmatrix}
 X_{3,n} & -X_{3,s} \\
 -X_{2,n} & X_{2,s}
\end{bmatrix}$$
• So the following equalities make sense:  

$$\begin{bmatrix}
 \begin{bmatrix}
 e_s \\
 e_n
\end{bmatrix} = \begin{bmatrix}
 X_{2,s} & X_{3,s} \\
 X_{2,n} & X_{3,n}
\end{bmatrix}
\begin{bmatrix}
 e_2 \\
 e_3
\end{bmatrix}, \text{ and } \begin{bmatrix}
 e_s \\
 e_n
\end{bmatrix} = \begin{bmatrix}
 X_{3,n} & -X_{2,n} \\
 -X_{3,s} & X_{2,s}
\end{bmatrix}
\begin{bmatrix}
 e_2 \\
 e_3
\end{bmatrix}.$$

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Solid Section Torsion



We invoke

$$\underline{e}_n = -X_{3,s}\underline{e}_2 + X_{2,s}\underline{e}_3$$
 here.

• Enforcing stress-free section boundary condtion leads to:

$$\begin{bmatrix} 0 & \sigma_{12} & \sigma_{13} \\ \sigma_{12} & 0 & 0 \\ \sigma_{13} & 0 & 0 \end{bmatrix} \underbrace{\begin{bmatrix} \hat{n} = -\underline{e}_n \\ 0 \\ X_{3,s} \\ -X_{2,s} \end{bmatrix}}_{\Longrightarrow \sigma_{12}X_{3,s} - \sigma_{13}X_{2,s} = 0} \\ (\phi_{,3}X_{3,s} + \phi_{2}X_{2,s}) = \phi_{,s} = 0$$

• That is, on the section-boundary, the stress function is constant, set to 0 w.l.o.g.:

$$\phi = constant^{0}$$
 on  $\Gamma$ .

• The strains are,

# 1.2. Displacement Formulation

Solid Section Torsion



- $$\begin{split} E_{11} &= u_{1,1} = 0 \\ E_{22} &= -\theta_{,2}X_3 = 0 \\ E_{33} &= \theta_{,3}X_2 = 0 \\ 2E_{23} &= \theta \theta = 0 \\ 2E_{12} &= u_{1,2} \theta_{,1}X_3 = \frac{\sigma_{12}}{G} = \frac{\phi_{,3}}{G} \\ 2E_{13} &= u_{1,3} + \theta_{,1}X_2 = \frac{\sigma_{13}}{G} = -\frac{\phi_{,2}}{G} \end{split}$$
- Differentiating the strain expressions for  $\sigma_{12}$  and  $\sigma_{13}$  above allows us to write:

$$\phi_{,kk} = -2G\theta_{,1} ,$$

which gives us the "constant" required for the Poisson problem from before (along with the B.C.  $\phi = 0 \text{ on } \Gamma$ ).

Solid Section Torsion

• The non-trivial shear strains are:

$$\sigma_{12} = \phi_{,3} = G(u_{1,2} - X_3\theta_{,1})$$
  
$$\sigma_{13} = -\phi_{,2} = G(u_{1,3} + X_2\theta_{,1})$$

• The moment about  $\underline{e}_1$  is

$$M_1 = \int_{\mathcal{S}} (X_2 \sigma_{13} - X_3 \sigma_{12}) dA \, dA$$

- Since  $\sigma_{12}$  and  $\sigma_{13}$  are expressed in terms of **kinematic quantities** as well as the **stress function**  $\phi$ , we will write down relationships with both before proceeding.
- It is also obvious that  $\phi_{,kk} = -2G\theta_{,1}$ implies

$$u_{1,kk} = 0$$



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 $\underline{e}_1$ 

# 1.3. Section Moment

Solid Section Torsion

• The non-trivial shear strains are:

$$\sigma_{12} = \phi_{,3} = G(u_{1,2} - X_3\theta_{,1})$$
  
$$\sigma_{13} = -\phi_{,2} = G(u_{1,3} + X_2\theta_{,1})$$

• The moment about  $\underline{e}_1$  is

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- Since  $\sigma_{12}$  and  $\sigma_{13}$  are expressed in terms of **kinematic quantities** as well as the **stress function**  $\phi$ , we will write down relationships with both before proceeding.
- It is also obvious that  $\phi_{,kk} = -2G\theta_{,1}$ implies

$$\rightarrow u_{1,kk} = 0.$$

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Let  $u_1 = u_1(X_1, X_2, X_3)$   $u_2 = -\theta X_3$   $u_3 = \theta X_2$   $u_3 = \theta X_2$ This is the governing equation in terms of the sectionaxial displacement field. Balaji, N. N. (AE, IITM)

 $\underline{e}_3$ 

Solid Section Torsion

#### In terms of stress function

$$M_1 = \int_{\mathcal{S}} (X_2 \sigma_{13} - X_3 \sigma_{12}) dA$$
$$= -\int_{\mathcal{S}} (\phi_{,2} X_2 + \phi_{,3} X_3) dA$$
$$M_1 = 2 \int_{\mathcal{S}} \phi dA.$$

In terms of kinematic description  

$$M_{1} = G \int_{S} (X_{2}u_{1,3} - X_{3}u_{1,2})dA$$

$$+ G \underbrace{\int_{S} (X_{2}^{2} + X_{3}^{2})dA \theta_{,1}}_{I_{11}}$$

$$= GI_{11}\theta_{,1} + G \int_{S} \epsilon_{1jk}X_{j}u_{1,k}dA$$

$$= GI_{11}\theta_{,1} + G \int_{S} \epsilon_{ijk}(X_{j}u_{1})_{,k}dA$$

$$- G \int_{S} \underbrace{\epsilon_{ijk}\delta_{jk}u_{1}dA}_{M_{1}} = GI_{11}\theta_{,1} + G \int_{\Gamma} \epsilon_{1jk}X_{j}n_{k}u_{1}d|s|$$

$$M_{1} = GI_{11}\theta_{,1} + G \int_{\Gamma} (\underline{X} \times \underline{n})_{1}u_{1}d|s|$$

Solid Section Torsion

#### In terms of stress function

$$M_1 = \int_{\mathcal{S}} (X_2 \sigma_{13} - X_3 \sigma_{12}) dA$$
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$$= GI_{11}\theta_{,1} + G \int \epsilon_{1jk}X_{j}u_{1,k} dA$$
This term is clearly  
zero for a perfectly  
circular section. What  
about other types?  

$$M_{1} = GI_{11}\theta_{,1} + G \int_{\Gamma} \epsilon_{1jk}X_{j}n_{k}u_{1}d|s|$$

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Solid Section Torsion

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Mot zero in the general case.  

$$M_{1} = GI_{11}\theta_{,1} + G \underbrace{\int_{\Gamma} (\underline{X} \times \underline{n})_{1}u_{1}d|s|}_{\Gamma}.$$

# 1.3. Section Moment: St. Venant's Warping Function

Solid Section Torsion

- For a "pure twist" condition, due to **translational symmetry**,  $u_1$  can not depend on  $X_1$ . It also makes sense that  $u_1$  has to be proportional to the twist  $\theta$  somehow.
- Since  $\theta$  depends on  $X_1$ , but  $\theta_{,1}$  is a constant, St. Venant introduced a warping function  $\psi(X_2, X_3)$  such that

$$u_1 = \theta_{,1} \psi(X_2, X_3)$$
.

• Under this definition, the effective moment  $M_1$  can be given as,

$$M_1 = G\underbrace{\left(I_{11} + \int_{\Gamma} (\underline{X} \times \underline{n})_1 \psi d|s|\right)}_{J} \theta_{,1} = GJ\theta_{,1}.$$

 $\bullet$  Alternatively, J can also be written as,

$$J = I_{11} + \int_{\mathcal{S}} X_2 \psi_{,3} - X_3 \psi_{,2} dA$$

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# 1.3. Section Moment: St. Venant's Warping Function

Solid Section Torsion

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• Al The product GJ is also known as **Torsional Rigidity** 

$$J = I_{11} + \int_{\mathcal{S}} X_2 \psi_{,3} - X_3 \psi_{,2} dA$$

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Solid Section Torsion

• The governing equations in terms of Prandtl Stress function is

$$\phi_{,kk} + 2G\theta_{,1} = 0, \qquad \phi = 0 \operatorname{on} \Gamma,$$

along with  $M_1 = 2 \int_{\mathcal{S}} \phi dA$ .

Solid Section Torsion

• The governing equations in terms of Prandtl Stress function is

$$\phi_{,kk} + 2G\theta_{,1} = 0, \qquad \phi = 0 \operatorname{on} \Gamma,$$

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Transverse Deflections of a Membrane under Isotropic Linear Tension Density T and Uniform Planar Load Density  $\overline{P}$ 

- The displacement field  $u_1 = 0$ ,  $u_2 = 0$ ,  $u_3 = w(X_1, X_2)$ • The strain Field  $E_{11} = \frac{w_{,1}^2}{2}$ ,  $E_{22} = \frac{w_{,2}^2}{2}$ ,  $2E_{12} = w_{,1}w_{,2}$ 
  - The Stress Field

$$\sigma_{11} = \frac{1}{t}T, \quad \sigma_{22} = \frac{1}{t}T.$$



Solid Section Torsion

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Transverse Deflections of a Membrane under Isotropic Linear Tension Density T and Uniform Planar Load Density  $\overline{P}$ 

• The displacement field

$$u_1 = 0, \quad u_2 = 0, \quad u_3 = w(X_1, X_2)$$

• The strain Field

$$E_{11} = \frac{w_{,1}^2}{2}, \quad E_{22} = \frac{w_{,2}^2}{2}, \quad 2E_{12} = w_{,1}w_{,2}$$

• The Stress Field

$$\sigma_{11} = \frac{1}{t}T, \quad \sigma_{22} = \frac{1}{t}T.$$

• Strain Energy Density (Integrated over thickness)

$$\mathcal{U} = \frac{1}{2} \left( w_{,1}^2 + w_{,2}^2 \right) T + Pw$$

• Equations of Motion <sup>*a*</sup>:  

$$\frac{\partial}{\partial X_k} \frac{\partial \mathcal{U}}{\partial w_{,k}} - \frac{\partial \mathcal{U}}{\partial w} = 0$$
:

$$T(w_{,11} + w_{,22}) - P = 0$$

aEuler-Ostrogradsky

Solid Section Torsion

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Transverse Deflections of a Membrane under Isotropic Linear Tension

**Density** T and **Un** The governing equations, therefore, are identical **Energy Density** to that of a **membrane** rated over thickness) • The displacem undergoing deformation  $u_1 = 0, \quad u_2 = 0$ under the action of a  $= \frac{1}{2} \left( w_{,1}^2 + w_{,2}^2 \right) T + Pw$ uniform area-load P. • The strain Fiere  $E_{11} = \frac{w_{,1}^2}{2}, \quad E_{22} = \frac{w_{,2}^2}{2}, \quad 2E_{12} = w_{,1}w_{,2} \bullet \text{Equations of Motion}^{a}: \\ \frac{\partial}{\partial X_{*}} \frac{\partial \mathcal{U}}{\partial w_{*}} - \frac{\partial \mathcal{U}}{\partial w} = 0:$ • The Stress Field  $\sigma_{11} = \frac{1}{4}T, \quad \sigma_{22} = \frac{1}{4}T.$  $\overline{T(w_{,11} + w_{,22})} - P = 0$ aEuler-Ostrogradsky

# 1.4. Membrane Analogy: Governing Equations of $u_1$ (Warping) Solid Section Torsion

• The governing equations in terms of  $u_1$  is the Laplace equation:

$$u_{1,kk} = 0,$$

and its boundary conditions (Neumann B.C.s) are written as (again based on zero traction at free end:

$$G \left\langle (u_{1,2} - X_3\theta_1)\underline{e}_2 + (u_{1,3} + X_2\theta_1)\underline{e}_3, \underline{e}_n \right\rangle = 0$$
  

$$\Longrightarrow \left\langle u_{1,2}\underline{e}_2 + u_{1,3}\underline{e}_3, X_{2,n}\underline{e}_2 + X_{3,n}\underline{e}_3 \right\rangle$$
  

$$-\theta_{,1} \left\langle X_3\underline{e}_2 - X_2\underline{e}_3, -X_{3,s}\underline{e}_2 + X_{2,s}\underline{e}_3 \right\rangle = 0$$
  

$$\Longrightarrow \left[ u_{1,n} = -\theta_{,1}\frac{d}{ds} \left( X_2^2 + X_3^2 \right) \right] = -\frac{\theta_{,1}}{2} \left( X_3 X_{2,n} - X_2 X_{3,n} \right).$$

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Solid Section Torsion Membrane Analogy

# 1.4. Membrane Analogy: Governing Equations of $u_1$ (Warping)

Solid Section Torsion

• The governing equations in terms of  $u_1$  is the Lanace equation: Note: We have used two different representations of  $\underline{e}_n$  here: and its boundary based on zero traction  $\underline{e}_n = X_{2,n}\underline{e}_2 + X_{3,n}\underline{e}_3$ , and  $\underline{e}_n = -X_{3,s}\underline{e}_2 + X_{2,s}\underline{e}_3$   $G \langle (u_{1,2} - X_3\theta_1)\underline{e}_2 + (u_{1,3} + X_2\theta_1)\underline{e}_3, \underline{e}_n \rangle = 0$  $\Longrightarrow \langle u_{1,2}\underline{e}_2 + u_{1,3}\underline{e}_3, X_{2,n}\underline{e}_2 + X_{3,n}\underline{e}_3 \rangle$ 

$$-\theta_{,1}\langle X_3\underline{e}_2 - X_2\underline{e}_3, -X_{3,s}\underline{e}_2 + X_{2,s}\underline{e}_3 \rangle = 0$$
  
$$\implies \boxed{u_{1,n} = -\theta_{,1}\frac{d}{ds} \left(X_2^2 + X_3^2\right)} = -\frac{\theta_{,1}}{2} \left(X_3 X_{2,n} - X_2 X_{3,n}\right).$$

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Solid Section Torsion

Equations in the Stress Function

$$\nabla^2 \phi = -2G\theta_{,1},$$
  

$$\phi = 0 \text{ on } \Gamma,$$
  

$$M_1 = 2 \int_{\mathcal{S}} \phi dA.$$

#### Equations in Warping

$$\begin{split} \nabla^2 u_1 &= 0, \\ \frac{\partial u_1}{\partial n} &= -\theta_{,1} \frac{d}{ds} \left( X_2^2 + X_3^2 \right) \text{ on } \Gamma. \\ M_1 &= G J \theta_{,1} \end{split}$$

#### Relating the two

Once we find φ, we can integrate the following to get u<sub>1</sub>:

$$\frac{1}{G}\phi_{,3} = u_{1,2} - X_3\theta_{,1}$$
$$-\frac{1}{G}\phi_{,2} = u_{1,3} + X_2\theta_{,1}$$

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# 1.5. Tutorial: Elliptical Section

Solid Section Torsion

• Let us consider an elliptical section and choose the stress function as

$$\phi = C\left(\frac{X_2^2}{a^2} + \frac{X_3^2}{b^2} - 1\right).$$

 $\bullet\,$  The Laplacian of  $\phi$  evaluates as,

$$\nabla^2 \phi = 2C \left( \frac{1}{a^2} + \frac{1}{b^2} \right) = -2G\theta_{,1} \implies C = -G\theta_{,1} \frac{a^2 b^2}{a^2 + b^2}.$$

• Let us first compute the total resultant twisting moment  $M_1$  that this represents:

$$M_{1} = 2 \int_{\mathcal{S}} \phi = 2C \left( \underbrace{\frac{1}{a^{2}} \int_{\mathcal{S}} X_{2}^{2} dA}_{\mathcal{S}} + \underbrace{\frac{1}{b^{2}} \int_{\mathcal{S}} X_{3}^{2} dA}_{\mathcal{S}} - \underbrace{\int_{\mathcal{S}} dA}_{\mathcal{S}} \right) = -C\pi ab$$
$$M_{1} = G \frac{\pi a^{3} b^{3}}{a^{2} + b^{2}} \theta_{,1}.$$

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# 1.5. Tutorial: Elliptical Section

Solid Section Torsion

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 $\bullet\,$  The Laplacian of  $\phi$  evaluates as,

$$\nabla^2 \phi = 2C \left( \frac{1}{a^2} + \frac{1}{b^2} \right) = -2G\theta_{,1} \implies C = -G\theta_{,1} \frac{a^2 b^2}{a^2 + b^2}.$$

• Let us first compute the total resultant twisting moment  $M_1$  that this represents:

$$M_{1} = 2 \int_{\mathcal{S}} \phi = 2C \left( \underbrace{\frac{1}{a^{2}} \int_{\mathcal{S}} X_{2}^{2} dA}_{\mathcal{S}} + \underbrace{\frac{1}{b^{2}} \int_{\mathcal{S}} X_{3}^{2} dA}_{\mathcal{S}} - \underbrace{\int_{\mathcal{S}} dA}_{\mathcal{S}} \right) = -C\pi ab$$

$$M_{1} = G \frac{\pi a^{3} b^{3}}{a^{2} + b^{2}} \theta_{,1} \cdot \frac{GJ}{\mathcal{S}} = G \frac{\pi a^{3} b^{3}}{a^{2} + b^{2}}$$
N. N. (AE, IITM) AS3020\* September 28, 2024 15/23

## 1.5. Tutorial: Elliptical Section

Solid Section Torsion

• For the axial deflection we have two equations (by equating shear stress expressions),

$$u_{1,2} = \theta_{,1}\psi_{,2} = -\frac{2a^2}{a^2 + b^2}\theta_{,1}X_3 + \theta_{,1}X_3 = -\frac{a^2 - b^2}{a^2 + b^2}\theta_{,1}X_3$$
$$u_{1,3} = \theta_{,1}\psi_{,3} = \frac{2b^2}{a^2 + b^2}\theta_{,1}X_2 - \theta_{,1}X_2 = -\frac{a^2 - b^2}{a^2 + b^2}\theta_{,1}X_2$$

• Integrating them separately we have,

$$u_1 = -\frac{a^2 - b^2}{a^2 + b^2} \theta_{,1} X_2 + f_1(X_3)$$
$$= -\frac{a^2 - b^2}{a^2 + b^2} \theta_{,1} X_2 + f_2(X_2)$$

•  $f_1$  and  $f_2$  have to be constant. Setting it to zero we have,

$$u_1 = -\frac{a^2 - b^2}{a^2 + b^2} \theta_{,1} X_2 X_3 = -\frac{a^2 - b^2}{G\pi a^3 b^3} M_1 X_2 X_3 \, .$$

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## 1.5. Tutorial

Solid Section Torsion

#### **Stress Function**





#### 1.5. Tutorial

Solid Section Torsion



#### **General Sections**

- Torsion is amenable to analysis when the solid section boundary can be expressed in closed form AND its Laplacian evaluates to a constant. (See Chapter 9 in [3])
- Every assumed form of \$\phi\$ will give us a warping field. For an application wherein the section warping is also constrained, this solution is not exact. (St. Venant's principle can be invoked, however).
- Several analytical techniques exist (check [3] and references therein).
- Fully numerical approaches are also possible, see the FreeFem scripts in the website.

# 2. Thin Section Torsion

Transformation of Displacement Field to Skin-local Coordinates

We will consider the bending-torsion combined displacement field:

 $u_2 = v - X_3 \theta$  $u_3 = w + X_3 \theta,$ 

and transform this to the skin local coordinate system.



• The section displacement field transforms as,

$$\begin{bmatrix} u_s \\ u_n \end{bmatrix} = \begin{bmatrix} X_{2,s} & X_{3,s} \\ X_{2,n} & X_{3,n} \end{bmatrix} \begin{bmatrix} u_2 \\ u_3 \end{bmatrix}$$
$$= \begin{bmatrix} X_{3,n} & -X_{2,n} \\ -X_{3,s} & X_{3,n} \end{bmatrix} \begin{bmatrix} u_2 \\ u_3 \end{bmatrix}.$$

 The tangential component of displacement along the boundary Γ can be written as,

$$u_{s} = X_{2,s}(v - X_{3}\theta) + X_{3,s}(w + X_{2}\theta)$$
  
=  $X_{2,s}v + X_{3,s}w + \theta \underbrace{(X_{3,s}X_{2} - X_{2,s}X_{3})}_{-X_{n}}$   
 $\Rightarrow \underbrace{u_{s} = p\theta + vX_{2,s} + wX_{3,s}}_{-X_{n}}.$ 

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# 2. Thin Section Torsion

Transformation of Displacement Field to Skin-local Coordinates

• The transformed displacement field combining bending and torsion is:

$$\begin{array}{l} u_1 &= -X_3 v' - X_2 w' + \theta' \psi \\ u_2 &= v - X_3 \theta \\ u_3 &= w + X_2 \theta \end{array} \right\} \implies \begin{array}{l} u_1 \quad (\text{unchanged}) \\ \Longrightarrow \quad u_s &= p\theta + v X_{2,s} + w X_{3,s} \\ u_n &= X_s \theta - v X_{3,s} + w X_{2,s} \end{array}$$

 $\bullet\,$  The shear strain along a thin section between the  $\underline{e}_1,\,\underline{e}_s$  directions is

$$\gamma_{1s} = u_{1,s} + u_{s,1} = u_{1,s} + p\theta' + X_{2,s}v' + X_{3,s}w' = \frac{\tau}{G} = \frac{q(s)}{Gt}.$$

• Integrating this, we get

$$\int_{0}^{s} \frac{q(x)}{Gt} dx = (u_{1}(s) - u_{1}(0)) + \theta' \int_{0}^{s} p dx + v' \int_{0}^{s} X_{2,x} dx + \int_{0}^{s} X_{3,x} dx$$
$$= (u_{1}(s) - u_{1}(0)) + \theta' 2\mathcal{A}_{sweep}(s) + v'(X_{2}(s) - X_{2}(0))$$
$$+ w'(X_{3}(s) - X_{3}(0)).$$

• Over a **complete closed section** we have,

$$\oint \frac{q(s)}{Gt} ds = 2\mathcal{A}\theta'$$

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## 2.1. Open Sections

Thin Section Torsion

#### 2.2. Closed Sections

Thin Section Torsion

#### 2.3. Combined Cells

Thin Section Torsion

#### References I

- C. T. Sun. Mechanics of Aircraft Structures, 2nd edition. Hoboken, N.J: Wiley, June 2006. ISBN: 978-0-471-69966-8 (cit. on p. 2).
- T. H. G. Megson. Aircraft Structures for Engineering Students, Elsevier, 2013. ISBN: 978-0-08-096905-3 (cit. on p. 2).
- M. H. Sadd. Elasticity: Theory, Applications, and Numerics, 2nd ed. Amsterdam; Boston: Elsevier/AP, 2009. ISBN: 978-0-12-374446-3 (cit. on pp. 28, 29).