

## <span id="page-0-0"></span>AS3020: Aerospace Structures Module 5: Torsion of Beams

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# <span id="page-2-0"></span>1. [Solid Section Torsion](#page-2-0)

Basic Setup



- We assume:
	- No direct stresses applied:
		- $\sigma_{11} = \sigma_{22} = \sigma_{33} = 0$
	- <sup>2</sup> Sections "rotate rigidly":
		- $\gamma_{23} = 0 \implies \sigma_{23} = 0.$
	- <sup>3</sup> Body is at equilibrium under constant torque applied at right end.
- We will denote the section by  $\mathcal S$ and the section-boundary by Γ.

<span id="page-3-0"></span>[Solid Section Torsion](#page-2-0)

• Since we assume  $\sigma_{11} = \sigma_{22} = \sigma_{33} = \sigma_{23} = 0$ , the equilibrium equations read,

$$
\sigma_{12,2} + \sigma_{13,3} = 0, \quad \sigma_{12,1} = 0, \quad \sigma_{13,1} = 0.
$$

• We introduce the **Prandtl Stress Function**  $\phi(X_2, X_3)$  (no dependence on  $X_1$ ) such that

$$
\sigma_{12} = \phi_{,3}, \quad \sigma_{13} = -\phi_{,2}.
$$

This satisfies equilibrium by definition.

- In terms of strains the above assumptions imply that we only have  $E_{12}$ and  $E_{13}$  active. **Recall** that Strain compatibility is  $\epsilon_{mjk}\epsilon_{nil}E_{ij,mn} = 0$ (see Module 3).
- The non-trivial compatibility equations read,

$$
\left.\begin{array}{l}\nE_{12,23} - E_{13,22} = 0 \\
E_{12,33} - E_{13,23} = 0\n\end{array}\right\} \implies \begin{array}{l}\n\phi_{,332} + \phi_{,222} = 0 \\
\phi_{,333} + \phi_{,322} = 0\n\end{array}\n\implies \boxed{\nabla^2 \phi = \text{constant}}.
$$

• This is known as the **Poisson's problem**. What about Boundary Conditions?

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• Since we assume  $\sigma_{11} = \sigma_{22} = \sigma_{33} = \sigma_{23} = 0$ , the equilibrium equations read,

$$
\sigma_{12,2} + \sigma_{13,3} = 0, \quad \sigma_{12,1} = 0, \quad \sigma_{13,1} = 0.
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- $\bullet$  In terms of strains the above assumptions imply t and  $E_{13}$  active. **Recall** that Strain compatibility (see Module 3).
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\phi_{,333} + \phi_{,322} & = 0\n\end{array} \implies
$$

$$
\implies \boxed{\nabla^2 \phi = \text{constant}}.
$$

Kinematic considerations will give us this "constant".

• This is known as the **Poisson's problem**. What about Boundary Conditions?

[Solid Section Torsion](#page-2-0)



• We derive the coordinate transformation on the boundary as follows:

$$
dX_2 \underline{e}_2 + dX_3 \underline{e}_3 = d\overline{s} \underline{e}_s + d\overline{n} \underline{e}_n
$$
  
\n
$$
\implies \begin{bmatrix} dX_2 \\ dX_3 \end{bmatrix} = \begin{bmatrix} \langle \underline{e}_2, \underline{e}_s \rangle & \langle \underline{e}_2, \underline{e}_n \rangle \\ \langle \underline{e}_3, \underline{e}_s \rangle & \langle \underline{e}_3, \underline{e}_n \rangle \end{bmatrix} \begin{bmatrix} ds \\ dn \end{bmatrix}
$$
  
\nand, 
$$
\begin{bmatrix} \underline{e}_s \\ \underline{e}_n \end{bmatrix} = \begin{bmatrix} \langle \underline{e}_2, \underline{e}_s \rangle & \langle \underline{e}_3, \underline{e}_s \rangle \\ \langle \underline{e}_2, \underline{e}_n \rangle & \langle \underline{e}_3, \underline{e}_n \rangle \end{bmatrix} \begin{bmatrix} \underline{e}_2 \\ \underline{e}_3 \end{bmatrix}
$$
  
\n
$$
= \begin{bmatrix} X_{2,s} & X_{3,s} \\ X_{2,n} & X_{3,n} \end{bmatrix} \begin{bmatrix} \underline{e}_2 \\ \underline{e}_3 \end{bmatrix}
$$

Considering only Cartesian transformations (inverse has to be transpose), we will also have

$$
\begin{bmatrix} \underline{e}_s \\ \underline{e}_n \end{bmatrix} = \begin{bmatrix} X_{3,n} & -X_{2,n} \\ -X_{3,s} & X_{2,s} \end{bmatrix} \begin{bmatrix} \underline{e}_2 \\ \underline{e}_3 \end{bmatrix}.
$$

[Solid Section Torsion](#page-2-0)



• We derive the coordinate transformation on the boundary as follows:

$$
dX_2 \underline{e}_2 + dX_3 \underline{e}_3 = d\underline{se}_s + d\underline{n} \underline{e}_n
$$
  
\n
$$
\implies \begin{bmatrix} dX_2 \\ dX_3 \end{bmatrix} = \begin{bmatrix} \langle \underline{e}_2, \underline{e}_s \rangle & \langle \underline{e}_2, \underline{e}_n \rangle \\ \langle \underline{e}_3, \underline{e}_s \rangle & \langle \underline{e}_3, \underline{e}_n \rangle \end{bmatrix} \begin{bmatrix} ds \\ dn \end{bmatrix}
$$
  
\nand, 
$$
\begin{bmatrix} \underline{e}_s \\ \underline{e}_n \end{bmatrix} = \begin{bmatrix} \langle \underline{e}_2, \underline{e}_s \rangle & \langle \underline{e}_3, \underline{e}_s \rangle \\ \langle \underline{e}_2, \underline{e}_n \rangle & \langle \underline{e}_3, \underline{e}_n \rangle \end{bmatrix} \begin{bmatrix} \underline{e}_2 \\ \underline{e}_3 \end{bmatrix}
$$
  
\n
$$
\implies \begin{bmatrix} X_{2,s} & X_{3,s} \\ X_{2,n} & X_{3,n} \end{bmatrix} \begin{bmatrix} \underline{e}_2 \\ \underline{e}_3 \end{bmatrix}
$$

Considering only Cartesian transformations (inverse has to be transpose), we will also have

$$
\mathbf{S}\begin{bmatrix} \underline{e}_s\\ \underline{e}_n \end{bmatrix} = \begin{bmatrix} X_{3,n} & -X_{2,n} \\ -X_{3,s} & X_{2,s} \end{bmatrix} \begin{bmatrix} \underline{e}_2 \\ \underline{e}_3 \end{bmatrix}
$$

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 $\leftarrow$   $\Box$ 

.

# 1.1. Derivation of Coordinate Transformation Relationships

[Stress Formulation](#page-3-0)

For cartesian transformations, the determinant has to be unity. So the inverse can be written as

$$
\underbrace{\begin{bmatrix} X_{2,s} & X_{3,s} \\ X_{2,n} & X_{3,n} \end{bmatrix}}_{\mathbb{T}^{-1}} = \underbrace{\begin{bmatrix} X_{3,n} & -X_{3,s} \\ -X_{2,n} & X_{2,s} \end{bmatrix}}_{Adj(\mathbb{T})}.
$$

Also, for cartesian transformations, the inverse has to be the transpose of the matrix. So we have

$$
\underbrace{\begin{bmatrix} X_{2,s} & X_{2,n} \\ X_{3,s} & X_{3,n} \end{bmatrix}}_{\mathbb{T}^T} = \underbrace{\begin{bmatrix} X_{3,n} & -X_{3,s} \\ -X_{2,n} & X_{2,s} \end{bmatrix}}_{\mathbb{T}^{-1}}.
$$
\n• So the following equalities make sense:\n
$$
\underbrace{\begin{bmatrix} \underline{e}_s \\ \underline{e}_n \end{bmatrix}}_{\text{Balaji, N. N. (AE, IITM)}} = \underbrace{\begin{bmatrix} X_{2,s} & X_{3,s} \\ X_{2,n} & X_{3,n} \end{bmatrix}}_{\text{ASO20*}} \underbrace{\begin{bmatrix} \underline{e}_s \\ \underline{e}_n \end{bmatrix}}_{\text{Sptember 28, 2024}} = \underbrace{\begin{bmatrix} X_{2,n} & X_{2,n} \\ -X_{3,s} & X_{2,s} \end{bmatrix}}_{\text{Sptember 28, 2024}}.
$$

[Solid Section Torsion](#page-2-0)



We invoke

$$
\underline{e}_n=-X_{3,s}\underline{e}_2+X_{2,s}\underline{e}_3
$$
 here.

Enforcing stress-free section boundary condtion leads to:



That is, on the section-boundary, the stress function is constant, set to  $0 \text{ w.l.o.g.}$ :

$$
\phi = \text{constant}^{\bullet 0} \quad \text{on } \Gamma.
$$

• The strains are,

# <span id="page-9-0"></span>1.2. [Displacement Formulation](#page-9-0)

[Solid Section Torsion](#page-2-0)



- $E_{11} = u_{1,1} = 0$  $E_{22} = -\theta_2 X_3 = 0$  $E_{33} = \theta_3 X_2 = 0$  $2E_{23} = \theta - \theta = 0$  $2E_{12} = u_{1,2} - \theta_{,1}X_3 = \frac{\sigma_{12}}{C}$  $\frac{\sigma_{12}}{G} = \frac{\phi_{,3}}{G}$ G  $2E_{13} = u_{1,3} + \theta_{,1}X_2 = \frac{\sigma_{13}}{G}$  $\frac{\sigma_{13}}{G} = -\frac{\phi_{,2}}{G}$ G
- Differentiating the strain expressions for  $\sigma_{12}$  and  $\sigma_{13}$  above allows us to write:

$$
\phi_{,kk} = -2G\theta_{,1},
$$

which gives us the "constant" required for the Poisson problem from before (along with the B.C.  $\phi = 0$  on  $\Gamma$ ).

<span id="page-10-0"></span>[Solid Section Torsion](#page-2-0)

The non-trivial shear strains are:

$$
\sigma_{12} = \phi_{,3} = G(u_{1,2} - X_3 \theta_{,1})
$$
  

$$
\sigma_{13} = -\phi_{,2} = G(u_{1,3} + X_2 \theta_{,1})
$$

The moment about  $\underline{e}_1$  is

$$
M_1 = \int_{\mathcal{S}} (X_2 \sigma_{13} - X_3 \sigma_{12}) dA.
$$

- Since  $\sigma_{12}$  and  $\sigma_{13}$  are expressed in terms of kinematic quantities as well as the stress function  $\phi$ , we will write down relationships with both before proceeding.
- It is also obvious that  $\phi_{.kk} = -2G\theta_{.1}$ implies

$$
u_{1,kk} = 0.
$$



[Solid Section Torsion](#page-2-0)

• The non-trivial shear strains are:

$$
\sigma_{12} = \phi_{,3} = G(u_{1,2} - X_3 \theta_{,1})
$$
  

$$
\sigma_{13} = -\phi_{,2} = G(u_{1,3} + X_2 \theta_{,1})
$$

The moment about  $\underline{e}_1$  is

$$
M_1 = \int_{\mathcal{S}} (X_2 \sigma_{13} - X_3 \sigma_{12}) dA.
$$

- Since  $\sigma_{12}$  and  $\sigma_{13}$  are expressed in terms of kinematic quantities as well as the stress function  $\phi$ , we will write down relationships with both before proceeding.
- It is also obvious that  $\phi_{.kk} = -2G\theta_{.1}$ implies

$$
\sqrt{u_{1,kk}=0}.
$$

 $\underline{e}_1$  $e_3$ Let  $u_1 = u_1(X_1, X_2, X_3)$  $u_2 = -\theta X_3$  $u_3 = \theta X_2$ This is the governing equation

in terms of the sectionaxial displacement field.

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[Solid Section Torsion](#page-2-0)

#### In terms of stress function

$$
M_1 = \int_{\mathcal{S}} (X_2 \sigma_{13} - X_3 \sigma_{12}) dA
$$
  
= 
$$
- \int_{\mathcal{S}} (\phi_{,2} X_2 + \phi_{,3} X_3) dA
$$
  

$$
M_1 = 2 \int_{\mathcal{S}} \phi dA.
$$

In terms of kinematic description  
\n
$$
M_1 = G \int_S (X_2 u_{1,3} - X_3 u_{1,2}) dA
$$
\n
$$
+ G \underbrace{\int_S (X_2^2 + X_3^2) dA \theta,1}_{I_{11}}
$$
\n
$$
= G I_{11} \theta,1 + G \int_S \epsilon_{1jk} X_j u_{1,k} dA
$$
\n
$$
= G I_{11} \theta,1 + G \int_S \epsilon_{ijk} (X_j u_{1}),k dA
$$
\n
$$
- G \int_S \underline{\epsilon_{ijk} \delta_{jk} u_1} dA
$$
\n
$$
M_1 = G I_{11} \theta,1 + G \int_{\Gamma} \epsilon_{1jk} X_j n_k u_1 d|s|
$$
\n
$$
M_1 = G I_{11} \theta,1 + G \int_{\Gamma} (\underline{X} \times \underline{n})_1 u_1 d|s|
$$

[Solid Section Torsion](#page-2-0)

#### In terms of stress function

$$
M_1 = \int_{\mathcal{S}} (X_2 \sigma_{13} - X_3 \sigma_{12}) dA
$$
  
= 
$$
- \int_{\mathcal{S}} (\phi_{,2} X_2 + \phi_{,3} X_3) dA
$$
  

$$
M_1 = 2 \int_{\mathcal{S}} \phi dA.
$$

In terms of kinematic description M<sup>1</sup> =G Z S (X2u1,<sup>3</sup> − X3u1,2)dA + G Z S (X 2 <sup>2</sup> + X 2 <sup>3</sup> )dA | {z } I11 θ,<sup>1</sup> =GI11θ,<sup>1</sup> + G Z S ϵ1jkXju1,kdA =GI11θ,<sup>1</sup> + G Z S ϵijk(Xju1),kdA − G Z S ✘ϵijk✘δ✘jku1dA M<sup>1</sup> =GI11θ,<sup>1</sup> + G Z Γ ϵ1jkXjnku1d|s| M<sup>1</sup> = GI11θ,<sup>1</sup> + G Z Γ (X × n)1u1d|s| . This term is clearly zero for a perfectly circular section. What about other types? Not zero in the general case.

[Solid Section Torsion](#page-2-0)

#### In terms of stress function

$$
M_1 = \int_{\mathcal{S}} (X_2 \sigma_{13} - X_3 \sigma_{12}) dA
$$
  
= 
$$
- \int_{\mathcal{S}} (\phi_{,2} X_2 + \phi_{,3} X_3) dA
$$
  

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In terms of kinematic description  
\n
$$
M_1 = G \int_S (X_2 u_{1,3} - X_3 u_{1,2}) dA
$$
\n
$$
+ G \underbrace{\int_S (X_2^2 + X_3^2) dA \theta_{,1}}_{I_{11}}
$$
\n
$$
= G I_{11} \theta_{,1} + G \underbrace{\int \epsilon_{1jk} X_j u_{1,k} dA}_{\text{2ero for a perfectly}} \text{This term is clearly zero for a perfectly circular section. What about other types?}
$$
\nNot zero in the general case.  
\n
$$
M_1 = G I_{11} \theta_{,1} + G \underbrace{\int_{\Gamma} (X \times n) u_{1} d|s}.
$$

# 1.3. [Section Moment:](#page-10-0) St. Venant's Warping Function

[Solid Section Torsion](#page-2-0)

- For a "pure twist" condition, due to **translational symmetry**,  $u_1$  can not depend on  $X_1$ . It also makes sense that  $u_1$  has to be proportional to the twist  $\theta$  somehow.
- Since  $\theta$  depends on  $X_1$ , but  $\theta_{,1}$  is a constant, St. Venant introduced a warping function  $\psi(X_2, X_3)$  such that

$$
u_1 = \theta_{,1}\psi(X_2,X_3).
$$

 $\bullet$  Under this definition, the effective moment  $M_1$  can be given as,

$$
M_1 = G\underbrace{\left(I_{11} + \int_{\Gamma} (\underline{X} \times \underline{n})_1 \psi d|s|\right)}_{J} \theta_{,1} = GJ\theta_{,1}.
$$

Alternatively,  $J$  can also be written as,

$$
J = I_{11} + \int_{\mathcal{S}} X_2 \psi_{,3} - X_3 \psi_{,2} dA
$$

# 1.3. [Section Moment:](#page-10-0) St. Venant's Warping Function

[Solid Section Torsion](#page-2-0)

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u_1=\theta_{,1}\psi(X_2,X_3).
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• Under this definition, the effective moment  $M_1$  can be given as,

$$
M_1 = G \underbrace{\left(I_{11} + \int_{\Gamma} (\underline{X} \times \underline{n})_1 \psi d |s|\right)}_{J} \theta_{,1} = G J \theta_{,1}.
$$

Al The product  $GJ$  is also known as **Torsional Rigidity** ۰

$$
J = I_{11} + \int_{\mathcal{S}} X_2 \psi_{,3} - X_3 \psi_{,2} dA
$$

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<span id="page-17-0"></span>[Solid Section Torsion](#page-2-0)

The governing equations in terms of Prandtl Stress function is

$$
\phi_{,kk} + 2G\theta_{,1} = 0, \qquad \phi = 0 \text{ on } \Gamma,
$$

along with  $M_1 = 2 \int_{\mathcal{S}} \phi dA$ .

[Solid Section Torsion](#page-2-0)

The governing equations in terms of Prandtl Stress function is

$$
\phi_{,kk} + 2G\theta_{,1} = 0, \qquad \phi = 0 \text{ on } \Gamma,
$$

along with  $M_1 = 2 \int_{\mathcal{S}} \phi dA$ .

Transverse Deflections of a Membrane under Isotropic Linear Tension Density  $T$  and Uniform Planar Load Density  $P$ 

The displacement field

$$
u_1 = 0, \quad u_2 = 0, \quad u_3 = w(X_1, X_2)
$$

- **The strain Field**  $E_{11} = \frac{w_{,1}^2}{2}$  $\frac{v_{1}^{2}}{2}, E_{22} = \frac{w_{12}^{2}}{2}$  $\frac{2^{s},2}{2}$ ,  $2E_{12} = w_{,1}w_{,2}$ uniform area-load P.
- The Stress Field

$$
\sigma_{11} = \frac{1}{t}T, \quad \sigma_{22} = \frac{1}{t}T.
$$



[Solid Section Torsion](#page-2-0)

The governing equations in terms of Prandtl Stress function is

$$
\phi_{,kk} + 2G\theta_{,1} = 0, \qquad \phi = 0 \text{ on } \Gamma,
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along with  $M_1 = 2 \int_{\mathcal{S}} \phi dA$ .

Transverse Deflections of a Membrane under Isotropic Linear Tension Density  $T$  and Uniform Planar Load Density  $P$ 

- The displacement field (Int  $u_1 = 0, \quad u_2 = 0, \quad u_3 = w(X_1, X_2)$  $\frac{\text{d}}{\text{d}x}$
- **a** The strain Field

$$
E_{11} = \frac{w_{,1}^2}{2}, \quad E_{22} = \frac{w_{,2}^2}{2}, \quad 2E_{12} = w_{,1}w_{,2}
$$

**o** The Stress Field

$$
\sigma_{11} = \frac{1}{t}T, \quad \sigma_{22} = \frac{1}{t}T.
$$

Strain Energy Density (Integrated over thickness) therefore, are identical

$$
\mathcal{U} = \frac{1}{2} \left( w_{,1}^2 + w_{,2}^2 \right) T + P w
$$

• Equations of Motion <sup>a</sup>:  
\n
$$
\frac{\partial}{\partial X_k} \frac{\partial U}{\partial w_{,k}} - \frac{\partial U}{\partial w} = 0:
$$

$$
T(w_{,11} + w_{,22}) - P = 0
$$

aEuler-Ostrogradsky

[Solid Section Torsion](#page-2-0)

The governing equations in terms of Prandtl Stress function is

$$
\phi_{,kk} + 2G\theta_{,1} = 0, \qquad \phi = 0 \text{ on } \Gamma,
$$

along with  $M_1 = 2 \int_{\mathcal{S}} \phi dA$ .

Transverse Deflections of a Membrane under Isotropic Linear Tension

**Density**  $T$  and  $\overline{\text{Un}}$  The governing equations,  $\bullet$  The displacem  $u_1 = 0, \quad u_2 = 0$  $\bullet$  The strain Fie $\frac{1}{2}$  $E_{11} = \frac{w_{,1}^2}{2}$  $\frac{v_{1}^{2}}{2}, E_{22} = \frac{w_{12}^{2}}{2}$  $\frac{\partial \zeta_2}{\partial z}$ ,  $2E_{12} = w_{,1}w_{,2}$  • Equations of Motion <sup>a</sup>:<br> $\frac{\partial}{\partial X_k} \frac{\partial U}{\partial w_{,k}} - \frac{\partial U}{\partial w} = 0$ : **o** The Stress Field  $\sigma_{11} = \frac{1}{1}$  $\frac{1}{t}T$ ,  $\sigma_{22} = \frac{1}{t}$  $\frac{1}{t}T.$ **Energy Density** rated over thickness)  $=\frac{1}{2}$  $\frac{1}{2}\left(w_{,1}^2+w_{,2}^2\right)T+Pw$  $T(w_{,11} + w_{,22}) - P = 0$ aEuler-Ostrogradsky therefore, are identical to that of a membrane undergoing deformation under the action of a uniform area-load P.

## 1.4. [Membrane Analogy:](#page-17-0) Governing Equations of  $u_1$ (Warping) [Solid Section Torsion](#page-2-0)

 $\bullet$  The governing equations in terms of  $u_1$  is the **Laplace equation:** 

$$
u_{1,kk}=0,
$$

and its boundary conditions (Neumann B.C.s) are written as (again based on zero traction at free end:

$$
G \langle (u_{1,2} - X_3 \theta_1) \underline{e}_2 + (u_{1,3} + X_2 \theta_1) \underline{e}_3, \underline{e}_n \rangle = 0
$$
  
\n
$$
\implies \langle u_{1,2} \underline{e}_2 + u_{1,3} \underline{e}_3, X_{2,n} \underline{e}_2 + X_{3,n} \underline{e}_3 \rangle
$$
  
\n
$$
- \theta_{,1} \langle X_{3} \underline{e}_2 - X_{2} \underline{e}_3, -X_{3,s} \underline{e}_2 + X_{2,s} \underline{e}_3 \rangle = 0
$$
  
\n
$$
\implies u_{1,n} = -\theta_{,1} \frac{d}{ds} \left( X_2^2 + X_3^2 \right) = -\frac{\theta_{,1}}{2} \left( X_3 X_{2,n} - X_2 X_{3,n} \right).
$$

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# 1.4. [Membrane Analogy:](#page-17-0) Governing Equations of  $u_1$ (Warping)

[Solid Section Torsion](#page-2-0)

 $\bullet$  The governing equations in terms of  $u_1$  is the **Lan** acceptace equation: ent representations of  $\underline{e}_n$  here: and its boundary  $e_n = X_{2,n}e_2 + X_{3,n}e_3$ , and  $e_n$  written as (again based on zero trad  $G\left\langle (u_{1,2}-X_3\theta_{,1})\underline{e}_2+(u_{1,3}+X_2\theta_{,1})\underline{e}_3,\underline{e}_n\right\rangle =0$  $\Longrightarrow \langle u_{1,2} \underline{e}_2 + u_{1,3} \underline{e}_3, X_{2,n} \underline{e}_2 + X_{3,n} \underline{e}_3 \rangle$  $- \theta_{,1} \langle X_3 \underline{e}_2 - X_2 \underline{e}_3, -X_3, \underline{e}_2 + X_2, \underline{e}_3 \rangle = 0$  $\Rightarrow$   $\left| u_{1,n} = -\theta_{,1} \frac{d}{dt} \right|$  $\frac{d}{ds} (X_2^2 + X_3^2) = -\frac{\theta_{,1}}{2}$  $\frac{1}{2}$   $(X_3X_{2,n}-X_2X_{3,n})$ . Note: We have used two differ- $\underline{e}_n = X_{2,n}\underline{e}_2 + X_{3,n}\underline{e}_3$ , and  $e_n = -X_{3,s}e_2 + X_{2,s}e_3$ 

[Solid Section Torsion](#page-2-0)

Equations in the Stress Function

$$
\nabla^2 \phi = -2G\theta_{,1},
$$
  
\n
$$
\phi = 0 \text{ on } \Gamma,
$$
  
\n
$$
M_1 = 2 \int_S \phi dA.
$$

#### Equations in Warping

$$
\nabla^2 u_1 = 0,
$$
  
\n
$$
\frac{\partial u_1}{\partial n} = -\theta_{,1} \frac{d}{ds} \left( X_2^2 + X_3^2 \right) \text{ on } \Gamma.
$$
  
\n
$$
M_1 = GJ\theta_{,1}
$$

#### Relating the two

 $\bullet$  Once we find  $\phi$ , we can integrate the following to get  $u_1$ :

$$
\frac{1}{G}\phi_{,3} = u_{1,2} - X_3\theta_{,1} - \frac{1}{G}\phi_{,2} = u_{1,3} + X_2\theta_{,1}
$$

# <span id="page-24-0"></span>1.5. [Tutorial:](#page-24-0) Elliptical Section

[Solid Section Torsion](#page-2-0)

Let us consider an elliptical section and choose the stress function as

$$
\phi = C \left( \frac{X_2^2}{a^2} + \frac{X_3^2}{b^2} - 1 \right).
$$

• The Laplacian of  $\phi$  evaluates as,

$$
\nabla^2 \phi = 2C \left( \frac{1}{a^2} + \frac{1}{b^2} \right) = -2G\theta_{,1} \implies C = -G\theta_{,1} \frac{a^2 b^2}{a^2 + b^2}.
$$

 $\bullet$  Let us first compute the total resultant twisting moment  $M_1$  that this represents:

$$
M_1 = 2 \int_{S} \phi = 2C \left( \frac{1}{a^2} \overbrace{\int_{S} X_2^2 dA + \frac{1}{b^2} \overbrace{\int_{S} X_3^2 dA - \int_{S} \pi a b}^{\frac{\pi a b^3}{4}} \right) = -C \pi a b
$$
\n
$$
M_1 = G \frac{\pi a^3 b^3}{a^2 + b^2} \theta, 1
$$
\nBalaji, N. N. (AE, IITM)

 $\Box$ 

# 1.5. [Tutorial:](#page-24-0) Elliptical Section

[Solid Section Torsion](#page-2-0)

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M_1 = 2 \int_{S} \phi = 2C \left( \frac{1}{a^2} \overbrace{\int_{S} X_2^2 dA + \frac{1}{b^2} \overbrace{\int_{S} X_3^2 dA - \int_{S} \pi ab}^{\frac{\pi ab^3}{4}} - \frac{\pi ab}{b^2}} \right) = -C \pi ab
$$
  
The torsional rigidity reads,  

$$
M_1 = G \frac{\pi a^3 b^3}{a^2 + b^2} \theta, \Gamma
$$

# 1.5. [Tutorial:](#page-24-0) Elliptical Section

[Solid Section Torsion](#page-2-0)

For the axial deflection we have two equations (by equating shear stress expressions),

$$
u_{1,2} = \theta_{,1}\psi_{,2} = -\frac{2a^2}{a^2 + b^2}\theta_{,1}X_3 + \theta_{,1}X_3 = -\frac{a^2 - b^2}{a^2 + b^2}\theta_{,1}X_3
$$
  

$$
u_{1,3} = \theta_{,1}\psi_{,3} = \frac{2b^2}{a^2 + b^2}\theta_{,1}X_2 - \theta_{,1}X_2 = -\frac{a^2 - b^2}{a^2 + b^2}\theta_{,1}X_2
$$

• Integrating them separately we have,

$$
u_1 = -\frac{a^2 - b^2}{a^2 + b^2} \theta_{,1} X_2 + f_1(X_3)
$$
  
= 
$$
-\frac{a^2 - b^2}{a^2 + b^2} \theta_{,1} X_2 + f_2(X_2)
$$

•  $f_1$  and  $f_2$  have to be constant. Setting it to zero we have,

$$
u_1 = -\frac{a^2 - b^2}{a^2 + b^2} \theta_{,1} X_2 X_3 = -\frac{a^2 - b^2}{G \pi a^3 b^3} M_1 X_2 X_3.
$$

## <span id="page-27-0"></span>1.5. [Tutorial](#page-24-0)

[Solid Section Torsion](#page-2-0)





## <span id="page-28-0"></span>1.5. [Tutorial](#page-24-0)

[Solid Section Torsion](#page-2-0)



#### General Sections

- Torsion is amenable to analysis when the solid section boundary can be expressed in closed form AND its Laplacian evaluates to a constant. (See Chapter 9 in [\[3\]](#page-34-3))
- $\bullet$  Every assumed form of  $\phi$  will give us a warping field. For an application wherein the section warping is also constrained, this solution is not exact. (St. Venant's principle can be invoked, however).
- Several analytical techniques exist (check [\[3\]](#page-34-3) and references therein).
- Fully numerical approaches are also possible, see the FreeFem scripts in the website.

€⊡

# <span id="page-29-0"></span>2. [Thin Section Torsion](#page-29-0)

Transformation of Displacement Field to Skin-local Coordinates

We will consider the bending-torsion combined displacement field:

> $u_2 = v - X_3 \theta$  $u_3 = w + X_3\theta,$

and transform this to the skin local coordinate system.



• The section displacement field transforms as,

$$
\begin{bmatrix} u_s \\ u_n \end{bmatrix} = \begin{bmatrix} X_{2,s} & X_{3,s} \\ X_{2,n} & X_{3,n} \end{bmatrix} \begin{bmatrix} u_2 \\ u_3 \end{bmatrix}
$$

$$
= \begin{bmatrix} X_{3,n} & -X_{2,n} \\ -X_{3,s} & X_{3,n} \end{bmatrix} \begin{bmatrix} u_2 \\ u_3 \end{bmatrix}.
$$

The tangential component of displacement along the boundary Γ can be written as,

$$
u_s = X_{2,s}(v - X_3\theta) + X_{3,s}(w + X_2\theta)
$$
  
=  $X_{2,s}v + X_{3,s}w + \theta \underbrace{(X_{3,s}X_2 - X_{2,s}X_3)}_{-X_n}$   

$$
\implies u_s = p\theta + vX_{2,s} + wX_{3,s}.
$$

# 2. [Thin Section Torsion](#page-29-0)

Transformation of Displacement Field to Skin-local Coordinates

• The transformed displacement field combining bending and torsion is:

$$
\begin{aligned}\nu_1 &= -X_3v' - X_2w' + \theta'\psi \\
u_2 &= v - X_3\theta \\
u_3 &= w + X_2\theta\n\end{aligned}\n\right\} \implies\n\begin{aligned}\nu_1 \qquad \text{(unchanged)} \\
u_2 &= p\theta + vX_{2,s} + wX_{3,s} \\
u_n &= X_s\theta - vX_{3,s} + wX_{2,s}\n\end{aligned}
$$

The shear strain along a thin section between the  $\underline{e}_1$ ,  $\underline{e}_s$  directions is

$$
\gamma_{1s} = u_{1,s} + u_{s,1} = u_{1,s} + p\theta' + X_{2,s}v' + X_{3,s}w' = \frac{\tau}{G} = \frac{q(s)}{Gt}.
$$

• Integrating this, we get

$$
\int_{0}^{s} \frac{q(x)}{Gt} dx = (u_1(s) - u_1(0)) + \theta' \int_{0}^{s} p dx + v' \int_{0}^{s} X_{2,x} dx + \int_{0}^{s} X_{3,x} dx
$$

$$
= (u_1(s) - u_1(0)) + \theta' 2A_{sweep}(s) + v'(X_2(s) - X_2(0))
$$

$$
+ w'(X_3(s) - X_3(0)).
$$

• Over a complete closed section we have,

$$
\boxed{\oint \frac{q(s)}{Gt} ds = 2\mathcal{A}\theta'}
$$

4 0 8

# <span id="page-31-0"></span>2.1. [Open Sections](#page-31-0)

[Thin Section Torsion](#page-29-0)

## <span id="page-32-0"></span>2.2. [Closed Sections](#page-32-0)

[Thin Section Torsion](#page-29-0)

## <span id="page-33-0"></span>2.3. [Combined Cells](#page-33-0)

[Thin Section Torsion](#page-29-0)

## <span id="page-34-0"></span>References I

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